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ICC-ES Evaluation Report

ESR-1940

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Reissued 01/2018
This report is subject to renewal 11/2020.

DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES
SECTION: 06 02 00—DESIGN INFORMATION

REPORT HOLDER:

APA- THE ENGINEERED WOOD ASSOCIATION

**7011 SOUTH 19TH STREET
TACOMA, WASHINGTON 98466**

EVALUATION SUBJECT:

GLUED-LAMINATED TIMBER COMBINATIONS AND THE GAP COMPUTER PROGRAM



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Reissued January 2018

This report is subject to renewal January 2020.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES
Section: 06 02 00—Design Information

REPORT HOLDER:

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EVALUATION SUBJECT:

GLUED-LAMINATED TIMBER COMBINATIONS AND THE GAP COMPUTER PROGRAM

ADDITIONAL LISTEES:

ANTHONY FOREST PRODUCTS CO.
309 NORTH WASHINGTON
EL DORADO, ARKANSAS 71730

CALVERT COMPANY, INC.
218 V STREET
VANCOUVER, WASHINGTON 98661

ROSBORO, LLC
POST OFFICE BOX 20
SPRINGFIELD, OREGON 97477

WESTERN STRUCTURES, INC.
POST OFFICE BOX 23355
EUGENE, OREGON 97402

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2015, 2012 and 2009 *International Building Code*® (IBC)
- 2015, 2012 and 2009 *International Residential Code*® (IRC)

Property evaluated:

Structural

2.0 USES

The GAP computer program is utilized to determine design stresses for the specific layups of glued-laminated timbers listed in Tables 1 and 2 of this report.

Glued-laminated timbers manufactured to the glued-laminated timber combinations or single grade layups that have been developed using the GAP program,

and that are produced at the facilities listed in Table 3, are recognized as being in compliance with the design parameters indicated in Section 3.0 of this report.

3.0 DESCRIPTION

The GAP computer program is based on the principles of ASTM D3737. It is an alternative method for determining associated allowable design stresses for a given layout combination of glued-laminated timber. The GAP computer program complies with the IBC and the IRC for allowable stress design. The design assumptions discussed in Sections 3.1 through 3.4 of this report are basic parameters utilized with the development of the allowable design stresses for the combinations listed in Table 1 or single grade layups listed in Table 2. See Section 5.4 for requirements applicable to these parameters.

3.1 Adhesive:

Face and end-joint bonding adhesives comply with ASTM D2559 for exterior or wet use.

3.2 End Joints:

End joints comply with ANSI A190.1 and ASTM D3737.

3.3 Lumber:

Lumber having a nominal thickness of 2 inches or less is glued-laminated into rectangular cross sections complying with industry standards for depth, width, and appearance. Lumber that is E-rated or visually graded complies with rules of applicable approved lumber grading agencies and the procedures set forth in the manufacturer's quality control documentation. Quality control for E-rating and beam fabrication is conducted under the supervision of an approved third-party inspection agency. Grade specifications are included in rules of the applicable approved lumber grading agencies and follow industry classifications and nomenclature as provided in the applicable code.

3.4 Layout:

Beams are fabricated in accordance with ANSI A190.1 using the grade combinations noted in Table 1 or single grade layups noted in Table 2 of this report. Combinations are in accordance with ASTM D3737 requirements. Resawn purlin beams, manufactured by ripping nominally 6-inch beams vertically through their depth into two members of equal width, are permitted to be produced from Canadian spruce-pine (CSP) and spruce-pine-fir (SPF) combinations in this width without any variation in basic grade description or layout procedures.

4.0 DESIGN

The design requirements of structural glued-laminated timber must comply with Section 2306 or 2307 of the IBC,

or Sections R502.2 and R802.2 of the IRC, as applicable. Modifications of values for duration of load must comply with the IBC or the IRC, as applicable.

5.0 CONDITIONS OF USE

The specific layups for the glued-laminated timbers described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 The application of the GAP computer program is limited to the layup combinations shown in Tables 1 or 2. Design stresses for normal conditions of loading must not exceed those set forth in Tables 1 or 2.
- 5.2 Design stresses for combinations noted in Table 1 are for members with four or more laminations stressed primarily in bending due to loads applied perpendicular to the wide faces of the laminations. Design values are included, however, for axial stresses and stresses from bending due to loads applied parallel to the wide faces of the laminations.
- 5.3 Design stresses for combinations noted in Table 2 are for members with two or more laminations stressed primarily axially or in bending due to loads applied parallel to the wide faces of the laminations. Design values are included, however, for stresses from bending due to loads applied perpendicular to the wide faces of the laminations.
- 5.4 The effects of checking of the members are outside the scope of this report.
- 5.5 Glued-laminated timber manufactured to the glued-laminated timber combinations or single grade

layups that have been developed using the GAP program, listed in Tables 1 and 2, and that are produced at the facilities listed in Table 3, are recognized as being in compliance with the design parameters indicated in Section 3.0 of this report.

Evaluation of glue-laminated timber manufactured in accordance with this report but produced by manufacturers not listed in Table 3 must be recognized in a current ICC-ES report as being in compliance with the design parameters indicated in Section 3.0 of this report.

- 5.6 The quality program for monitoring the use of the GAP computer program must be in accordance with "Quality Control Requirements for the GAP Computer Program," dated July 26, 2006.

6.0 EVIDENCE SUBMITTED

- 6.1 Program Guide for the GAP Computer Program.
- 6.2 Data in accordance with ASTM D3737.
- 6.3 Quality system documentation.

7.0 IDENTIFICATION

Each glued-laminated beam manufactured using layup combinations determined in accordance with this report and produced at the facilities listed in Table 3 must be identified with the ICC-ES evaluation report number (ESR-1940).

Table 1 (Continued) – Reference Design Values for Structural Glued Laminated Softwood Timber Combinations^(a)
 (Members stressed primarily in bending) (Tabulated design values are for normal load duration and dry service conditions.)

| Combination Symbol | Species ^(b) Outer/Core | Bending About X-X Axis | | | | Bending About Y-Y Axis | | | | Axially Loaded | | Fasteners | | | | | |
|------------------------------------|------------------------------------|---|--|---|--|--|------------------------------------|--|---|--|--|----------------------|-------------------------------|--------------------------------------|-----------|------|---------|
| | | (Loaded Perpendicular to Wide Faces of Laminations) | | (Loaded Parallel to Wide Faces of Laminations) | | Modulus of Elasticity ^(c) | | Modulus of Elasticity ^(c) | | Tension Parallel to Grain | Compression Parallel to Grain | | | | | | |
| | | Extreme Fiber in Bending ^(d) | Top of Beam Stressed in Tension (Negative Bending) | Bottom of Beam Stressed in Tension (Positive Bending) | Fiber in Bending ^(d) | Shear Parallel to Grain | Compression Perpendicular to Grain | Shear Parallel to Grain | Compression Perpendicular to Grain | Extreme Fiber in Bending ^(d) | Shear Parallel to Grain | | Compression Parallel to Grain | Specific Gravity for Fastener Design | | | |
| F _{bx} ⁺ (psi) | F _{bx} ⁻ (psi) | F _{bx} (psi) | F _{vx} ^(e) (psi) | E _{x,true} (10 ⁶ psi) | E _{x,app} (10 ⁶ psi) | E _{x,min} (10 ⁶ psi) | F _{by} (psi) | F _{xy} ^(d,e) (psi) | E _{x,true} (10 ⁶ psi) | E _{x,app} (10 ⁶ psi) | E _{x,min} (10 ⁶ psi) | F _t (psi) | F _c (psi) | Top or Bottom Face | Side Face | | |
| 26F-V1 | SP/SP | 2600 | 2000 | 740 | 300 | 1.9 | 1.8 | 0.95 | 1700 | 650 | 260 | 1.6 | 1.7 | 0.85 | 1150 | 1600 | 0.55 |
| 26F-V2 | SP/SP | 2600 | 2100 | 740 | 300 | 2.0 | 1.9 | 1.00 | 1950 | 740 | 260 | 1.8 | 1.9 | 0.95 | 1300 | 1850 | 0.55 |
| 26F-V3 | SP/SP | 2600 | 2100 | 740 | 300 | 2.0 | 1.9 | 1.00 | 1950 | 740 | 260 | 1.8 | 1.9 | 0.95 | 1250 | 1800 | 0.55 |
| 26F-V3M1 ^(f) | SP/SP | 2600 | 2100 | 740 | 300 | 2.0 | 1.9 | 1.00 | 1950 | 650 | 260 | 1.8 | 1.9 | 0.95 | 1250 | 1800 | 0.55 |
| 26F-V3M2 ^(f) | SP/SP | 2600 | 2100 | 740 | 250 | 2.0 | 1.9 | 1.00 | 1950 | 650 | 260 | 1.8 | 1.9 | 0.95 | 1250 | 1800 | 0.55 |
| 26F-V4 | SP/SP | 2600 | 2600 | 740 | 300 | 2.0 | 1.9 | 1.00 | 1700 | 650 | 260 | 1.8 | 1.9 | 0.95 | 1200 | 1600 | 0.55 |
| 26F-V4M1 ^(f) | SP/SP | 2600 | 2600 | 740 | 300 | 2.0 | 1.9 | 1.00 | 1700 | 650 | 260 | 1.8 | 1.9 | 0.95 | 1200 | 1600 | 0.55 |
| 26F-V4M2 ^(f) | SP/SP | 2600 | 2600 | 740 | 250 | 2.0 | 1.9 | 1.00 | 1700 | 650 | 260 | 1.8 | 1.9 | 0.95 | 1200 | 1600 | 0.55 |
| 28F-E1 | SP/SP | 2800 | 2300 | 805 | 300 | 2.2 ^(g) | 2.1 ^(h) | 1.11 ⁽ⁱ⁾ | 1600 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1300 | 1850 | 0.55 |
| 28F-E1M1 | SP/SP | 2800 | 2300 | 805 | 300 | 2.2 | 2.1 | 1.11 | 1600 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1300 | 1850 | 0.55 |
| 28F-E2 | SP/SP | 2800 | 2800 | 805 | 300 | 2.2 ^(g) | 2.1 ^(h) | 1.11 ⁽ⁱ⁾ | 2000 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1300 | 1850 | 0.55 |
| 28F-E2M1 | SP/SP | 2800 | 2800 | 805 | 300 | 2.2 | 2.1 | 1.11 | 2000 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1300 | 1850 | 0.55 |
| 30F-E1 ^(m) | SP/SP | 3000 | 2400 | 805 | 300 | 2.2 ^(g) | 2.1 ^(h) | 1.11 ⁽ⁱ⁾ | 1750 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1250 | 1750 | 0.55 |
| 30F-E1M1 ^(m) | SP/SP | 3000 | 2400 | 805 | 300 | 2.2 | 2.1 | 1.11 | 1750 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1250 | 1750 | 0.55 |
| 30F-E1M2 ⁽ⁿ⁾ | LVL/SP | 3000 ^(o) | 2400 | 650 ^(p) | 300 | 2.2 | 2.1 | 1.11 | 1750 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1250 | 1750 | 0.50 |
| 30F-E2 ^(m) | SP/SP | 3000 | 3000 | 805 | 300 | 2.2 ^(g) | 2.1 ^(h) | 1.11 ⁽ⁱ⁾ | 1750 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1350 | 1750 | 0.55 |
| 30F-E2M1 ^(m) | SP/SP | 3000 | 3000 | 805 | 300 | 2.2 | 2.1 | 1.11 | 1750 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1350 | 1750 | 0.55 |
| 30F-E2M2 ⁽ⁿ⁾ | LVL/SP | 3000 ^(o) | 3000 ^(o) | 650 ^(p) | 300 | 2.2 | 2.1 | 1.11 | 1750 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1350 | 1750 | 0.55 |
| 30F-E2M3 ⁽ⁿ⁾ | LVL/SP | 3000 ^(o) | 3000 ^(o) | 650 ^(p) | 300 | 2.2 | 2.1 | 1.11 | 1750 | 650 | 260 | 1.7 | 1.8 | 0.90 | 1350 | 1750 | 0.50 |
| Wet-use factors | | 0.8 | | 0.53 | | 0.833 | | 0.833 | 0.8 | 0.53 | 0.875 | 0.833 | 0.8 | 0.73 | 0.8 | 0.73 | See NDS |

For S1: 1 psi = 6.895 Pa

- (a) The combinations in this table are applicable to members consisting of 4 or more laminations and are intended primarily for members stressed in bending due to loads applied perpendicular to the wide faces of the laminations. However, design values are tabulated for loading both perpendicular and parallel to the wide faces of the laminations. For combinations and design values applicable to members loaded primarily axially or parallel to the wide faces of the laminations, see Table 2. For members of 2 or 3 laminations, see Table 2. The tabulated design values are for dry conditions of use. For wet conditions of use, multiply the tabulated values by the factors shown at the bottom of the table. The tabulated design values are for normal duration of loading. For other durations of loading, see applicable building code.
- (b) The symbols used for species are AC = Alaska cedar, CSP = Canadian spruce-pine, DF = Douglas fir-larch, ES = Eastern spruce, HF = Hem-fir, POC = Port Orford cedar, SP = Spruce-pine-fir, and SW = Softwood species.
- (c) The tabulated design values in bending, F_{bx}, are based on members 5-1/8 inches in width by 12 inches in depth by 21 feet in length. For members with a larger volume, F_{bx} must be multiplied by a volume factor, C_v, determined in accordance with applicable building code. The tabulated F_{bx} values require the use of special tension laminations. If these special tension laminations are omitted, the F_{bx} values must be multiplied by 0.75 for members greater than or equal to 15 inches or by 0.85 for members less than 15 inches in depth. 20F-E/ES1 does not require special tension laminations.
- (d) The design values for shear, F_{xy}, and F_{xy} shall be decreased by multiplying by a factor of 0.72 for non-prismatic members, notched members, and for all members subject to impact or cyclic loading. The reduced design value shall be used for design of members at connections that transfer shear by mechanical fasteners. The reduced design value shall also be used for determination of design values for radial tension and torsion. F_{xy} and F_{xy} values do not include adjustments for checking.
- (e) Design values are for timbers with laminations made from a single piece of lumber across the width or multiple pieces that have been edge bonded. For timber manufactured from multiple piece laminations (across width) that are not edge-bonded, value shall be multiplied by 0.4 for members with 5, 7, or 9 laminations or by 0.5 for all other members. This reduction shall be cumulative with the adjustment in footnote (d).
- (f) See Section 2.5 of ANSI 117 (www.apawood.org) for the E_{x,true}, E_{x,app}, and E_{x,min}.
- (g) The values of F_{bx} were calculated based on members 12 inches in depth (bending about Y-Y axis). For depths other than 12 inches, the F_{bx} values are permitted to be increased by multiplying by the size factor, (12/d)^{0.5}, where d is the beam depth in inches. When d is less than 3 inches, use the size adjustment factor for 3 inches.
- (h) The beam depth limitation is as follows: 20F-E/ES1: 15 inches; 24F-V5M2/DF: 27 inches; 24F-V5M3/DF and 24F-V/DF1: 24 inches; 26F-E/DF1: 24 inches; 26F-E/DF1M1: 9-1/2, 11-7/8, 14, and 16 inches.
- (i) 20F-E/SPF1 is limited to 1-1/2 to 3-1/2 inches in width, and 7-1/2, 9, 9-1/2, 11-7/8, and 14 inches in depth. 24F-E/SP1 is limited to 9-1/2, 11-7/8, 14, 16, and 18 inches in depth.
- (j) When containing wane, this combination must be used in dry conditions only. In this case, wet-use factors must not be applied. Because of the wane, this combination is available only for an industrial appearance characteristic. If wane is omitted, these restrictions must not apply. This combination is limited to 9 to 20 laminations in depth except for 16F-V5M1/SP, which contains a maximum of 1/6 wane on each side and must be 4 laminations or more in depth.
- (k) For 26F-E/DF1, the F_{bx} value is permitted to be increased to 2,200 psi for beam depths less than 16 inches. For 24F-V/DF1, the F_{bx} value is permitted to be increased to 1,300 psi for beam depths of at least 10-1/2 inches.
- (l) This combination must be manufactured from either 24F-V4WS, 24F-V5M1/WS, 24F-V5M2/WS, 24F-V5M3/WS, 24F-E15M1/WS, 24F-E/SPF4, or 24F-V3SP, and is intended primarily for use in header applications.
- (m) This layout combination is limited to nominal 6 inches or less in width. In addition, 30F-E1M1/SP and 30F-E2M1/SP are limited to 18 inches or less in depth.
- (n) The beam depth is limited to 16 inches or less for 30F-E2M2/SP, and 30 inches or less for 30F-E1M2/SP and 30F-E2M3/SP. The tension lamination requirements for these layouts must not be omitted.
- (o) The tabulated design values in bending, F_{bx}, must be multiplied by a volume factor, C_v, determined in accordance with applicable building code using 1/10 as the exponent.
- (p) The allowable compressive stress perpendicular to grain of the beam must be increased to the published allowable compressive stress perpendicular to grain of the outermost laminated veneer lumber.
- (q) For 26F and 30F members with more than 15 laminations, E_{x,true} = 2.1 x 10⁶ psi, E_{x,app} = 2.0 x 10⁶ psi, and E_{x,min} = 1.06 x 10⁶ psi.
- (r) This combination may contain lumber with wane. If lumber with wane is used, the design value for shear parallel to grain, F_{xy}, shall be multiplied by 0.67 if wane is allowed on both sides. If wane is limited to one side, F_{xy} must be multiplied by 0.83. This reduction is cumulative with the adjustment in footnote (d).
- (s) This combination may contain wane. If wane lumber is used, F_{xy} must be multiplied by 0.67 if wane is allowed on both sides. If wane is limited to one side, F_{xy} must be multiplied by 0.83. This reduction is cumulative with the adjustment in footnote (d).

Table 2 – Reference Design Values for Structural Glued Laminated Softwood Timber
(Members stressed primarily in axial tension or compression) (Tabulated design values are for normal load duration and dry service conditions.)

| Combination Symbol | Species | Grade | All Loading | | | | Axially Loaded | | | | Bending about Y-Y Axis Loaded Parallel to Wide Faces of Laminations | | | | Bending About X-X Axis Loaded Perpendicular to Wide Faces of Laminations | | Fasteners Specific Gravity for Fastener Design | |
|---------------------------------|---------|-------|--|--|---|--|--|--|--|---|--|--------------------------|---|---------|---|---|---|--------------------------|
| | | | Modulus of Elasticity (10 ⁶ psi) | | Compression Perpendicular to Grain F _c (psi) | Tension Parallel to Grain F _t (psi) | Compression Parallel to Grain F _c (psi) | 2 or More Lam-nations F _t (psi) | 4 or More Lam-nations F _c (psi) | 2 or 3 Lam-nations F _c (psi) | Bending | | Shear Parallel to Grain ^(a) F _v (psi) | Bending | Shear Parallel to Grain ^(a) F _v (psi) | 2 Lam-nations to 15 in. Deep ^(c) F _{bx} (psi) | | F _{vx} (psi) |
| | | | E _{axial} (10 ⁶ psi) | 0.95 E _{axial} (10 ⁶ psi) | | | | | | | E _{axial min} (10 ⁶ psi) | F _{bx} (psi) | | | | | | |
| Visually Graded Western Species | | | | | | | | | | | | | | | | | | |
| 1 | DF | L3 | 1.6 | 1.5 | 0.79 | 560 | 950 | 1550 | 1250 | 1450 | 1000 | 1250 | 230 | 1250 | 265 | 265 | 0.50 | |
| 2 | DF | L2 | 1.7 | 1.6 | 0.85 | 560 | 1250 | 1950 | 1600 | 1800 | 1300 | 1700 | 230 | 1700 | 265 | 265 | 0.50 | |
| 3 | DF | L2D | 2.0 | 1.9 | 1.00 | 650 | 1450 | 2300 | 1900 | 2100 | 1550 | 2000 | 230 | 2000 | 265 | 265 | 0.50 | |
| 5 | DF | L1 | 2.1 | 2.0 | 1.06 | 650 | 1650 | 2400 | 2100 | 2400 | 1800 | 2200 | 230 | 2200 | 265 | 265 | 0.50 | |
| 22* | SW | L3 | 1.1 | 1.0 | 0.53 | 315 | 525 | 850 | 725 | 700 | 575 | 725 | 170 | 725 | 195 | 195 | 0.35 | |
| 70 | AC | L2 | 1.4 | 1.3 | 0.69 | 470 | 975 | 1450 | 1450 | 1400 | 1000 | 1350 | 230 | 1350 | 265 | 265 | 0.46 | |
| Visually Graded Southern Pine | | | | | | | | | | | | | | | | | | |
| 47 | SP | N2M12 | 1.5 | 1.4 | 0.74 | 650 | 1200 | 1900 | 1150 | 1750 | 1300 | 1550 | 260 | 1400 | 300 | 300 | 0.55 | |
| 48 | SP | N2D12 | 1.8 | 1.7 | 0.90 | 740 | 1400 | 2200 | 1350 | 2000 | 1800 | 1800 | 260 | 1800 | 300 | 300 | 0.55 | |
| 49 | SP | N1M16 | 1.8 | 1.7 | 0.90 | 650 | 1350 | 2100 | 1450 | 1950 | 1750 | 1750 | 260 | 1800 | 300 | 300 | 0.55 | |
| 50 | SP | N1D14 | 2.0 | 1.9 | 1.00 | 740 | 1550 | 2300 | 1700 | 2300 | 2100 | 2100 | 260 | 2100 | 300 | 300 | 0.55 | |
| WeUse factors | | | 0.9333 | | 0.53 | 0.8 | 0.73 | 0.8 | 0.8 | 0.8 | 0.8 | 0.8 | 0.875 | 0.8 | 0.875 | 0.875 | See NDS | |

For S1: 1 psi = 6.895 Pa

- (a) For members with 2 or 3 laminations, the shear design value for transverse loads parallel to the wide faces of the laminations, F_v, shall be reduced by multiplying by a factor of 0.84 or 0.95, respectively.
- (b) The shear design value for transverse loads applied parallel to the wide faces of the laminations, F_v, shall be multiplied by 0.4 for members with 5, 7, or 9 laminations manufactured from multiple piece laminations (across width) that are not edge bonded. The shear design value, F_v, shall be multiplied by 0.5 for all other members manufactured from multiple piece laminations with unbonded edge joints. This reduction shall be cumulative with the adjustment in footnote (a).
- (c) The design values for shear, F_{vx} and F_{vy}, shall be decreased by multiplying by a factor of 0.72 for non-prismatic members, notched members, and for all members subject to impact or cyclic loading. The reduced design value shall be used for design of members at connections that transfer shear by mechanical fasteners. The reduced design value shall also be used for determination of design values for radial tension and torsion.
- (d) The tabulated F_{vx} values are for members without special tension lams up to 15 inches in depth. If the member depth is greater than 15 inches without special tension lams, the tabulated F_{vx} values must be multiplied by a factor of 0.88. If special tension lams are used, the tabulated F_{vx} values are permitted to be increased by a factor of 1.18 regardless of the member depth.
- (e) When Western Cedars, Western Cedars (North), Western Woods, and Redwood (open grain) are used in combinations for Softwood Species (SW), the design value for modulus of elasticity shall be reduced by 100,000 psi. When Coast Sitka Spruce, Coast Species, Western White Pine, and Eastern White Pine are used in combinations for Softwood Species (SW) tabulated design values for shear parallel to grain, F_{vx} and F_{vy}, shall be reduced by 10 psi, before applying any other adjustments.

Table 3 – Manufacturing Locations

| Manufacturer | Location |
|-----------------------------|---|
| Anthony Forest Products Co. | 256 Cooper Drive, El Dorado, AR 71730 |
| Anthony Forest Products Co. | 256 Edison Road, Washington, GA 30676 |
| Calvert Company, Inc. | 218 V Street, Vancouver, WA 98661 |
| Calvert Company, Inc. | 3559 Truman Road, Washougal, WA 98671 |
| Rosboro | 22833 Vaughn Road, Veneta, OR 97487 |
| Rosboro | 2509 Main Street, Springfield, OR 97477 |
| Western Structures, Inc. | 1381 Bailey Hill Road, Eugene, OR 97402 |